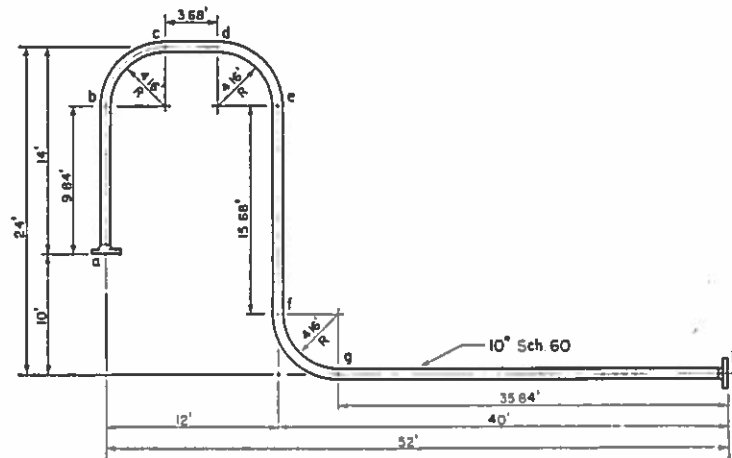


SINGLE PLANE SYSTEM CONTAINING CIRCULAR ARCS



Given: A 10 inch piping system in accordance with the sketch shown above.

Maximum Operating Pressure 400 psi
Maximum Operating Temperature 750° F
Piping Specification A.S.T.M. A-106 Grade A

Data:

$t = 0.500$ inches	} page 2 and 14
= schedule 60	
$I_P = 212$ inches ⁴	} page 14
$S_m = 39.43$ inches ³	
$k_{\text{bend}} = 1.74$	
$i_{\text{bend}} = 1.00$	
$c_{\text{at } 750^\circ} = 996$	page 11
$S_A = 17,675$ psi	page 3

Find: Reaction forces F_x and F_y at point h . (At point a reaction forces are equal and opposite.)

Reaction moments at points a and h .

Amount and location of Maximum Bending Stress, s_B .

Solution: Determine the location of the centroid and calculate the line inertias in the same manner as outlined on page 50 except that the flexibility factor k must be included for all curved segments of the system.

Solve for F_x and F_y using the equations shown on page 50:

$$L_x = 40 + 12 = 52 \text{ feet}$$

$$L_y = 24 - 14 = 10 \text{ feet}$$

$$F_x = \frac{21,369 \times 52 + 10,457 \times 10}{9415 \times 21,369 - (10,457)^2} \times 996 \times 212 = 2795 \text{ pounds}$$

$$F_y = \frac{9415 \times 10 + 10,457 \times 52}{9415 \times 21,369 - (10,457)^2} \times 996 \times 212 = 1467 \text{ pounds}$$

Pass these reaction forces through the centroid. The product of these forces and their respective distances from any point gives the bending moment at that point. Assume counterclockwise rotation as + and clockwise rotation as -.

Solve for reaction moments at points a and h as follows:

$$M_{\text{at } a} = -(2795 \times 0.40) + (1467 \times 17.37) = +24,364 \text{ ft lb}$$

$$M_{\text{at } h} = +(2795 \times 9.60) - (1467 \times 34.63) = -23,970 \text{ ft lb}$$

F , the resultant of F_x and F_y , when passed through the centroid gives the position of the neutral axis. The maximum bending moment occurs at that point which is furthest from the neutral axis. A scale drawing will show that this point is located between f and g at a normal distance of 9.4 feet from the neutral axis. The bending moment and bending stress in this curved section of pipe are determined as follows:

$$M = \sqrt{(2795)^2 + (1467)^2} \times 9.4 = 29,672 \text{ ft lb}$$

$$= 29,672 \times 12 = 356,064 \text{ inch pounds}$$

$$s_B = \frac{M}{S_m} i = \frac{356,064}{39.43} \times 1.00 = 9030 \text{ psi}$$

The maximum bending moment in straight pipe occurs at point g .

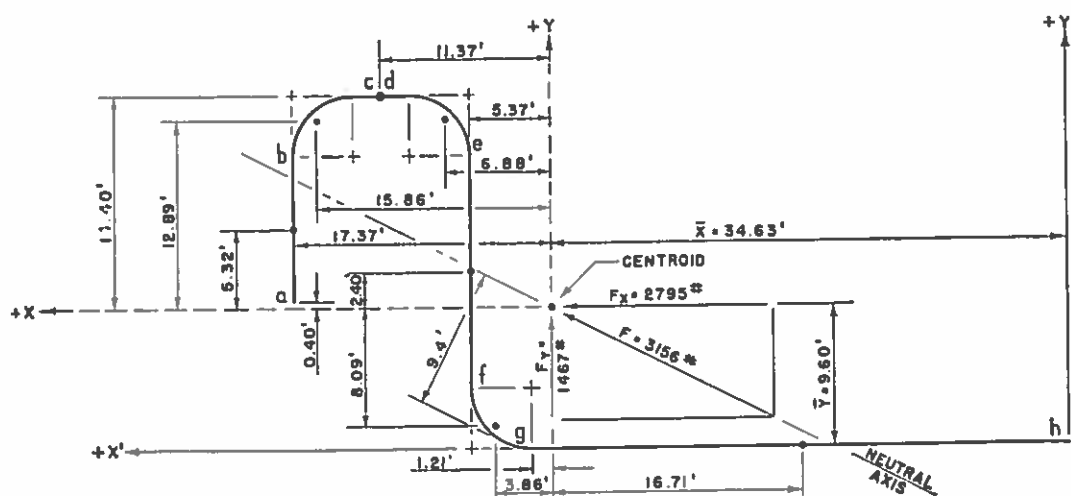
$$M_{\text{at } g} = +(2795 \times 9.60) + (1467 \times 1.21) = +28,607 \text{ ft lb}$$

$$= 28,607 \times 12 = 343,284 \text{ inch pounds}$$

$$s_B = \frac{M}{S_m} = \frac{343,607}{39.43} = 8714 \text{ psi}$$

The maximum bending stress is 9030 psi, occurring in member fg and is less than the allowable stress range of 22,500 psi.

EXPANSION AND STRESSES



To find c.g. of each segment see page 46.

$ab = 9.84$ ft
 $cd = 3.68$ ft
 $ef = 15.68$ ft
 $gh = 35.84$ ft

bc
 de
 fg } Radius of bends = 4.16 ft

Centroid (calculated with origin at point h)

	Eq. No.	Length L , Ft	x'	Lx'	y'	Ly'
ab	I	9.84	$+52$	$+ 511$	$+14.92$	$+147$
bc	III	$1.57 \times 1.74 \times 4.16 = 11.37$	$+50.49$	$+ 574.1$	$+22.49$	$+255.7$
cd	I	3.68	$+46$	$+ 169.2$	$+24.0$	$+ 88.4$
de	III	$1.57 \times 1.74 \times 4.16 = 11.37$	$+41.51$	$+ 472$	$+22.49$	$+255.7$
ef	I	15.68	$+40.0$	$+ 627$	$+12.0$	$+188$
fg	III	$1.57 \times 1.74 \times 4.16 = 11.37$	$+38.49$	$+ 437.6$	$+ 1.51$	$+ 17.2$
gh	I	35.84	$+17.92$	$+ 643$	0	0
		$\Sigma L = 99.15$		$\Sigma Lx' = +3433.9$		$\Sigma Ly' = +952.0$
		$\bar{x} = \frac{3433.9}{99.15} = +34.63$ ft			$\bar{y} = \frac{952.0}{99.15} = +9.60$ ft	
I_{xy}						
ab	VI	$9.84 \times 17.37 \times 5.32$				$= + 909$
bc	X A	$-1.74(0.137 \times 4.16^3) + 1.57 \times 1.74 \times 4.16 \times 15.86 \times 12.89$				$= + 2307$
cd	VI	$3.68 \times 11.37 \times 14.40$				$= + 603$
de	X B	$+1.74(0.137 \times 4.16^3) + 1.57 \times 1.74 \times 4.16 \times 6.88 \times 12.89$				$= + 1025$
ef	VI	$15.68 \times 5.37 \times 2.40$				$= + 202$
fg	X B	$+1.74(0.137 \times 4.16^3) + 1.57 \times 1.74 \times 4.16 \times 3.86(-8.09)$				$= - 338$
gh	VI	$35.84(-16.71)(-9.60)$				$= + 5749$
						$I_{xy} = +10457$
I_z						
ab	XIV B	$\frac{(9.84)^3}{12} + 9.84(5.32)^2$				$= 358$
bc	XVII A	$1.74(0.149 \times 4.16^3) + 1.57 \times 1.74 \times 4.16(12.89)^2$				$= 1908$
cd	XIV A	$3.68(14.40)^2$				$= 763$
de	XVII A	$1.74(0.149 \times 4.16^3) + 1.57 \times 1.74 \times 4.16(12.89)^2$				$= 1908$
ef	XIV B	$\frac{(15.68)^3}{12} + 15.68(2.40)^2$				$= 412$
fg	XVII A	$1.74(0.149 \times 4.16^3) + 1.57 \times 1.74 \times 4.16(8.09)^2$				$= 763$
gh	XIV A	$35.84(9.60)^2$				$= 3303$
						$I_z = 9415$
I_y						
ab	XIV A	$9.84(17.37)^2$				$= 2969$
bc	XVII B	$1.74(0.149 \times 4.16^3) + 1.57 \times 1.74 \times 4.16(15.86)^2$				$= 2879$
cd	XIV B	$\frac{(3.68)^3}{12} + 3.68(11.37)^2$				$= 480$
de	XVII B	$1.74(0.149 \times 4.16^3) + 1.57 \times 1.74 \times 4.16(6.88)^2$				$= 557$
ef	XIV A	$15.68(5.37)^2$				$= 452$
fg	XVII B	$1.74(0.149 \times 4.16^3) + 1.57 \times 1.74 \times 4.16(3.86)^2$				$= 188$
gh	XIV B	$\frac{(35.84)^3}{12} + 35.84(16.71)^2$				$= 13844$
						$I_y = 21369$

For equation reference numbers see pages 46 to 50.