APPENDIX A NOAA ATLAS 14 INFORMATION

APPENDIX A

Below is the depth-duration-frequency and intensity-duration-frequency data for Alpine City.



POINT PRECIPITATION FREQUENCY ESTIMATES FROM NOAA ATLAS 14



Utah 40.474 N 111.756 W 5209 feet

from "Precipitation-Frequency Atlas of the United States" NOAA Atlas 14, Volume 1, Version 4
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NOAA, National Weather Service, Silver Spring, Maryland, 2006
Extracted: Thu Jul 23 2009

Precipitation Frequency Estimates (inches)

| AEP* (1-in- Y) | <u>5</u> <u>min</u> | <u>10</u> <u>min</u> | <u>15</u> <u>min</u> | 30 min | <u>60</u> <u>min</u> | 120 min | <u>3 hr</u> | <u>6 hr</u> | <u>12 hr</u> | <u>24 hr</u> | <u>48 hr</u> | 4 day | <u>7 day</u> |
|----------------------|------------------------|-------------------------|-------------------------|-----------|-------------------------|------------|-------------|-------------|--------------|--------------|--------------|-------|--------------|
| 2 | 0.16 | 0.24 | 0.30 | 0.41 | 0.50 | 0.63 | 0.74 | 1.01 | 1.33 | 1.58 | 1.99 | 2.45 | 3.02 |
| 5 | 0.24 | 0.36 | 0.44 | 0.60 | 0.74 | 0.88 | 1.00 | 1.32 | 1.73 | 2.05 | 2.59 | 3.22 | 3.95 |
| 10 | 0.30 | 0.45 | 0.56 | 0.76 | 0.94 | 1.09 | 1.21 | 1.55 | 2.02 | 2.37 | 3.00 | 3.75 | 4.59 |
| 25 | 0.39 | 0.60 | 0.74 | 1.00 | 1.23 | 1.41 | 1.53 | 1.88 | 2.42 | 2.78 | 3.52 | 4.45 | 5.43 |
| 50 | 0.48 | 0.73 | 0.91 | 1.22 | 1.51 | 1.71 | 1.80 | 2.14 | 2.74 | 3.09 | 3.93 | 4.99 | 6.08 |
| 100 | 0.58 | 0.88 | 1.09 | 1.47 | 1.82 | 2.05 | 2.14 | 2.44 | 3.08 | 3.40 | 4.34 | 5.55 | 6.74 |
| 200 | 0.70 | 1.06 | 1.32 | 1.77 | 2.20 | 2.45 | 2.53 | 2.78 | 3.45 | 3.72 | 4.75 | 6.13 | 7.42 |
| 500 | 0.89 | 1.35 | 1.68 | 2.26 | 2.80 | 3.11 | 3.18 | 3.41 | 4.01 | 4.15 | 5.32 | 6.92 | 8.35 |
| 1000 | 1.07 | 1.62 | 2.01 | 2.71 | 3.35 | 3.71 | 3.78 | 3.98 | 4.48 | 4.48 | 5.75 | 7.55 | 9.08 |

^{*} These precipitation frequency estimates are based on an <u>annual maxima series</u>. **AEP** is the Annual Exceedance Probability. Please refer to <u>NOAA Atlas 14 Document</u> for more information. NOTE: Formatting forces estimates near zero to appear as zero.

Precipitation Intensity Estimates (in/hr)

| AEP* (1-in- Y) | <u>5 min</u> | <u>10</u> <u>min</u> | <u>15</u> <u>min</u> | 30 min | <u>60</u> <u>min</u> | 120 min | <u>3 hr</u> | <u>6 hr</u> | <u>12 hr</u> | <u>24 hr</u> | <u>48 hr</u> | 4 day | <u>7 day</u> |
|----------------------|--------------|-------------------------|-------------------------|-----------|-------------------------|------------|-------------|-------------|--------------|--------------|--------------|-------|--------------|
| 2 | 1.91 | 1.46 | 1.20 | 0.81 | 0.50 | 0.31 | 0.25 | 0.17 | 0.11 | 0.07 | 0.04 | 0.03 | 0.02 |
| 5 | 2.82 | 2.15 | 1.78 | 1.20 | 0.74 | 0.44 | 0.33 | 0.22 | 0.14 | 0.09 | 0.05 | 0.03 | 0.02 |
| 10 | 3.57 | 2.72 | 2.25 | 1.51 | 0.94 | 0.55 | 0.40 | 0.26 | 0.17 | 0.10 | 0.06 | 0.04 | 0.03 |
| 25 | 4.71 | 3.58 | 2.96 | 1.99 | 1.23 | 0.71 | 0.51 | 0.31 | 0.20 | 0.12 | 0.07 | 0.05 | 0.03 |
| 50 | 5.76 | 4.38 | 3.62 | 2.44 | 1.51 | 0.85 | 0.60 | 0.36 | 0.23 | 0.13 | 0.08 | 0.05 | 0.04 |
| 100 | 6.97 | 5.30 | 4.38 | 2.95 | 1.82 | 1.02 | 0.71 | 0.41 | 0.26 | 0.14 | 0.09 | 0.06 | 0.04 |
| 200 | 8.38 | 6.38 | 5.27 | 3.55 | 2.20 | 1.23 | 0.84 | 0.46 | 0.29 | 0.16 | 0.10 | 0.06 | 0.04 |
| 500 | 10.69 | 8.13 | 6.72 | 4.52 | 2.80 | 1.55 | 1.06 | 0.57 | 0.33 | 0.17 | 0.11 | 0.07 | 0.05 |
| 1000 | 12.80 | 9.74 | 8.05 | 5.42 | 3.35 | 1.86 | 1.26 | 0.66 | 0.37 | 0.19 | 0.12 | 0.08 | 0.05 |

^{*} These precipitation frequency estimates are based on an <u>annual maxima series.</u> **AEP** is the Annual Exceedance Probability. Please refer to <u>NOAA Atlas 14 Document</u> for more information. NOTE: Formatting forces estimates near zero to appear as zero.

APPENDIX B STORM DISTRIBUTIONS

APPENDIX B STORM DISTRIBUTIONS

Below are the 10-year storm distributions for the 3-, 6-, and 24-hour storm durations. The total precipitation was obtained from the data shown in Appendix A. In order to apply these storm durations for other storm frequencies, multiply the incremental precipitation values by the ratio of the new storm frequency total depth to the 10-year total depth. The Farmer-Fletcher 3-hour modified storm distribution is the one exception to this rule. Below is an explanation of how that storm distribution was developed.

Salt Lake County developed the modified version of the Farmer-Fletcher distribution by nesting the one-hour (quartile 1) Farmer-Fletcher storm distribution, within the three hour period. The difference between the three-hour and the one-hour rainfall depths is divided equally and is distributed over the first 30 minutes of the storm and from hour 1.5 to 3.0 (see Table B-1).

Table B-1
Farmer-Fletcher Modified 3-Hour
Storm Distribution

| Time (min) | Precipitation (Inches) |
|---------------|---------------------------|
| 0 | 0.000 |
| 5 | 0.011 |
| 10 | 0.011 |
| 15 | 0.011 |
| 20 | 0.011 |
| 25 | 0.011 |
| 30 | 0.011 |
| 35 | 0.268* |
| 40 | 0.212* |
| 45 | 0.148* |
| 50 | 0.094* |
| 55 | 0.056* |
| 60 | 0.043* |
| 65 | 0.032* |
| 70 | 0.024* |
| 75 | 0.019* |
| 80 | 0.017* |
| 85 | 0.015* |
| 90 | 0.012* |

| Time (min) | Precipitation (Inches) |
|---------------|---------------------------------------|
| 95 | 0.011 |
| 100 | 0.011 |
| 105 | 0.011 |
| 110 | 0.011 |
| 115 | 0.011 |
| 120 | 0.011 |
| 125 | 0.011 |
| 130 | 0.011 |
| 135 | 0.011 |
| 140 | 0.011 |
| 145 | 0.011 |
| 150 | 0.011 |
| 155 | 0.011 |
| 160 | 0.011 |
| 165 | 0.011 |
| 170 | 0.011 |
| 175 | 0.011 |
| 180 | 0.011 |
| | · · · · · · · · · · · · · · · · · · · |

Total: 1.21

^{*} Nested 1-hour storm distribution

Table B-2 NOAA Atlas 14 General Precipitation Area 6-Hour Storm Distribution

| Time | Precipitation |
|-------|---------------|
| (min) | (Inches) |
| 0 | 0.000 |
| 15 | 0.065 |
| 30 | 0.057 |
| 45 | 0.053 |
| 60 | 0.051 |
| 75 | 0.073 |
| 90 | 0.101 |
| 105 | 0.085 |
| 120 | 0.071 |
| 135 | 0.085 |
| 150 | 0.078 |
| 165 | 0.078 |
| 180 | 0.074 |
| 195 | 0.073 |
| 210 | 0.067 |
| 225 | 0.060 |
| 240 | 0.056 |
| 255 | 0.064 |
| 270 | 0.064 |
| 285 | 0.065 |
| 300 | 0.059 |
| 315 | 0.047 |
| 330 | 0.043 |
| 345 | 0.042 |
| 360 | 0.040 |
| | |

Total:

Table B-3 SCS Type II 24-Hour Storm Distribution

| Time | Precipitation |
|---------|---------------|
| (hours) | (Inches) |
| 0.0 | 0.000 |
| 0.5 | 0.013 |
| 1.0 | 0.013 |
| 1.5 | 0.013 |
| 2.0 | 0.014 |
| 2.5 | 0.014 |
| 3.0 | 0.015 |
| 3.5 | 0.016 |
| 4.0 | 0.016 |
| 4.5 | 0.017 |
| 5.0 | 0.018 |
| 5.5 | 0.019 |
| 6.0 | 0.020 |
| 6.5 | 0.021 |
| 7.0 | 0.023 |
| 7.5 | 0.025 |
| 8.0 | 0.027 |
| 8.5 | 0.030 |
| 9.0 | 0.033 |
| 9.5 | 0.037 |
| 10.0 | 0.043 |
| 10.5 | 0.055 |
| 11.0 | 0.073 |
| 11.5 | 0.114 |
| 12.0 | 0.900 |

| Time (hours) | Precipitation (Inches) |
|-----------------|---------------------------|
| 12.5 | 0.170 |
| 13.0 | 0.088 |
| 13.5 | 0.063 |
| 14.0 | 0.049 |
| 14.5 | 0.043 |
| 15.0 | 0.037 |
| 15.5 | 0.033 |
| 16.0 | 0.030 |
| 16.5 | 0.027 |
| 17.0 | 0.025 |
| 17.5 | 0.023 |
| 18.0 | 0.022 |
| 18.5 | 0.020 |
| 19.0 | 0.019 |
| 19.5 | 0.018 |
| 20.0 | 0.017 |
| 20.5 | 0.016 |
| 21.0 | 0.015 |
| 21.5 | 0.015 |
| 22.0 | 0.014 |
| 22.5 | 0.014 |
| 23.0 | 0.013 |
| 23.5 | 0.013 |
| 24.0 | 0.013 |
| Total: | 2.37 |

APPENDIX C TR-55 INFORMATION

Table 2-2a Runoff curve numbers for urban areas 1/

| Cover description | | | Curve nu hydrologic | umbers for soil group | |
|---|-------------------|---|------------------------|--------------------------|----------|
| | Average percent | | | | |
| Cover type and hydrologic condition in | npervious area 2/ | A | В | \mathbf{C} | D |
| Fully developed urban areas (vegetation established) | | | | | |
| Open space (lawns, parks, golf courses, cemeteries, etc.) 3/: | | | | | |
| Poor condition (grass cover < 50%) | <mark></mark> | 68 | 79 | 86 | 89 |
| Fair condition (grass cover 50% to 75%) | | 49 | <mark>69</mark> | <mark>79</mark> | 84 |
| Good condition (grass cover > 75%) | <mark></mark> | <mark>39</mark> | 61 | 74 | 80 |
| Impervious areas: | | | | | |
| Paved parking lots, roofs, driveways, etc. | | | | | |
| (excluding right-of-way) | •••• | 98 | 98 | 98 | 98 |
| Streets and roads: | | | | | |
| Paved; curbs and storm sewers (excluding | | | | | |
| right-of-way) | | 98 | 98 | 98 | 98 |
| Paved; open ditches (including right-of-way) | | 83 | 89 | 92 | 93 |
| Gravel (including right-of-way) | | 76 | 85 | 89 | 91 |
| Dirt (including right-of-way) | ••• | 72 | 82 | 87 | 89 |
| Western desert urban areas: | | - | _ | - | |
| Natural desert landscaping (pervious areas only) 4 | <mark></mark> | 63 | 77) | 85 | 88 |
| Artificial desert landscaping (impervious weed barrier, | | | | | |
| desert shrub with 1- to 2-inch sand or gravel mulch | | 0.0 | 0.0 | 0.0 | 0.0 |
| and basin borders) | <mark></mark> | 96 | 96 | 96 | 96 |
| Urban districts: | OF | 00 | 00 | 0.4 | 05 |
| Commercial and business | | 89 | 92 | 94 | 95 |
| Industrial | 72 | 81 | 88 | 91 | 93 |
| Residential districts by average lot size: | 65 | 77 | 85 | 90 | 92 |
| 1/8 acre or less (town houses) | | $\begin{array}{c} 77 \\ 61 \end{array}$ | 09 75 | 90 83 | 92 87 |
| 1/4 acre | | 57 | 73 72 | 81 | 86 |
| 1/2 acre | | 54 | 70 | 80 | 85 |
| 1/2 acre | | 54 51 | 68 | 79 | 84 |
| 2 acres | | 46 | 65 | 77 | 82 |
| 2 acres | 12 | 40 | 00 | 11 | 02 |
| Developing urban areas | | | | | |
| Newly graded areas | | | | | |
| (pervious areas only, no vegetation) 5/ | | 77 | 86 | 91 | 94 |
| Idle lands (CN's are determined using cover types | | | | | |
| similar to those in table 2-2c). | | | | | |

¹ Average runoff condition, and $I_a = 0.2S$.

² The average percent impervious area shown was used to develop the composite CN's. Other assumptions are as follows: impervious areas are directly connected to the drainage system, impervious areas have a CN of 98, and pervious areas are considered equivalent to open space in good hydrologic condition. CN's for other combinations of conditions may be computed using figure 2-3 or 2-4.

³ CN's shown are equivalent to those of pasture. Composite CN's may be computed for other combinations of open space cover type.

⁴ Composite CN's for natural desert landscaping should be computed using figures 2-3 or 2-4 based on the impervious area percentage (CN = 98) and the pervious area CN. The pervious area CN's are assumed equivalent to desert shrub in poor hydrologic condition.

⁵ Composite CN's to use for the design of temporary measures during grading and construction should be computed using figure 2-3 or 2-4 based on the degree of development (impervious area percentage) and the CN's for the newly graded pervious areas.

 Table 2-2b
 Runoff curve numbers for cultivated agricultural lands \underline{V}

| | Cover description | | | Curve num hydrologic s | | |
|--------------|----------------------------|--------------|----|---------------------------|----------|----|
| | cover description | Hydrologic | | 11, 01 010 610 0 | on group | |
| Cover type | Treatment 2/ | condition 3/ | A | В | С | D |
| Fallow | Bare soil | _ | 77 | 86 | 91 | 94 |
| | Crop residue cover (CR) | Poor | 76 | 85 | 90 | 93 |
| | | Good | 74 | 83 | 88 | 90 |
| Row crops | Straight row (SR) | Poor | 72 | 81 | 88 | 91 |
| - | | Good | 67 | 78 | 85 | 89 |
| | SR + CR | Poor | 71 | 80 | 87 | 90 |
| | | Good | 64 | 75 | 82 | 85 |
| | Contoured (C) | Poor | 70 | 79 | 84 | 88 |
| | | Good | 65 | 75 | 82 | 86 |
| | C + CR | Poor | 69 | 78 | 83 | 87 |
| | | Good | 64 | 74 | 81 | 85 |
| | Contoured & terraced (C&T) | Poor | 66 | 74 | 80 | 82 |
| | | Good | 62 | 71 | 78 | 81 |
| | C&T+ CR | Poor | 65 | 73 | 79 | 81 |
| | | Good | 61 | 70 | 77 | 80 |
| Small grain | SR | Poor | 65 | 76 | 84 | 88 |
| | | Good | 63 | 75 | 83 | 87 |
| | SR + CR | Poor | 64 | 75 | 83 | 86 |
| | | Good | 60 | 72 | 80 | 84 |
| | C | Poor | 63 | 74 | 82 | 85 |
| | | Good | 61 | 73 | 81 | 84 |
| | C + CR | Poor | 62 | 73 | 81 | 84 |
| | | Good | 60 | 72 | 80 | 83 |
| | C&T | Poor | 61 | 72 | 79 | 82 |
| | | Good | 59 | 70 | 78 | 81 |
| | C&T+ CR | Poor | 60 | 71 | 78 | 81 |
| | | Good | 58 | 69 | 77 | 80 |
| Close-seeded | SR | Poor | 66 | 77 | 85 | 89 |
| or broadcast | _ | Good | 58 | 72 | 81 | 85 |
| legumes or | C | Poor | 64 | 75 | 83 | 85 |
| rotation | | Good | 55 | 69 | 78 | 83 |
| meadow | C&T | Poor | 63 | 73 | 80 | 83 |
| | | Good | 51 | 67 | 76 | 80 |

 $^{^{1}}$ Average runoff condition, and I_a =0.2S

Poor: Factors impair infiltration and tend to increase runoff.

Good: Factors encourage average and better than average infiltration and tend to decrease runoff.

² Crop residue cover applies only if residue is on at least 5% of the surface throughout the year.

 $^{^3}$ Hydraulic condition is based on combination factors that affect infiltration and runoff, including (a) density and canopy of vegetative areas, (b) amount of year-round cover, (c) amount of grass or close-seeded legumes, (d) percent of residue cover on the land surface (good \geq 20%), and (e) degree of surface roughness.

 $\textbf{Table 2-2c} \qquad \text{Runoff curve numbers for other agricultural lands } \underline{1}{}^{\underline{1}}$

| Cover description | | Curve numbers for hydrologic soil group | | | | |
|--|-------------------------|---|----|----|----|--|
| Cover type | Hydrologic condition | A | В | С | D | |
| Pasture, grassland, or range—continuous | Poor | 68 | 79 | 86 | 89 | |
| forage for grazing. 2/ | Fair | 49 | 69 | 79 | 84 | |
| Totage for grazing. | Good | 39 | 61 | 74 | 80 | |
| Meadow—continuous grass, protected from grazing and generally mowed for hay. | _ | 30 | 58 | 71 | 78 | |
| Brush—brush-weed-grass mixture with brush | Poor | 48 | 67 | 77 | 83 | |
| the major element. ³ ⁄ | Fair | 35 | 56 | 70 | 77 | |
| | Good | 30 4/ | 48 | 65 | 73 | |
| Woods—grass combination (orchard | Poor | 57 | 73 | 82 | 86 | |
| or tree farm). 5/ | Fair | 43 | 65 | 76 | 82 | |
| , | Good | 32 | 58 | 72 | 79 | |
| Woods. 6/ | Poor | 45 | 66 | 77 | 83 | |
| | Fair | 36 | 60 | 73 | 79 | |
| | Good | 30 4/ | 55 | 70 | 77 | |
| Farmsteads—buildings, lanes, driveways, and surrounding lots. | _ | 59 | 74 | 82 | 86 | |

¹ Average runoff condition, and $I_a = 0.2S$.

² *Poor:* <50%) ground cover or heavily grazed with no mulch.

Fair: 50 to 75% ground cover and not heavily grazed.

Good: > 75% ground cover and lightly or only occasionally grazed.

³ *Poor*: <50% ground cover.

Fair: 50 to 75% ground cover.

Good: >75% ground cover.

⁴ Actual curve number is less than 30; use CN = 30 for runoff computations.

⁵ CN's shown were computed for areas with 50% woods and 50% grass (pasture) cover. Other combinations of conditions may be computed from the CN's for woods and pasture.

⁶ Poor: Forest litter, small trees, and brush are destroyed by heavy grazing or regular burning.

Fair: Woods are grazed but not burned, and some forest litter covers the soil.

Good: Woods are protected from grazing, and litter and brush adequately cover the soil.

 $\textbf{Table 2-2d} \qquad \text{Runoff curve numbers for arid and semiarid rangelands } \bot$

| Cover description | | | Curve nu hydrologi | mbers for c soil group | |
|--|---------------------------------------|-----------|-----------------------|---------------------------|-----------------|
| Cover type | Hydrologic condition ^{2/} | A 3/ | В | C | D |
| Herbaceous—mixture of grass, weeds, and | Poor | | 80 | 87 | 93 |
| low-growing brush, with brush the | Fair | | 71 | 81 | 89 |
| minor element. | Good | | 62 | 74 | 85 |
| Oak-aspen—mountain brush mixture of oak brush, | Poor | | 66 | 74 | <mark>79</mark> |
| aspen, mountain mahogany, bitter brush, maple, | Fair | | 48 | 57 | 63 |
| and other brush. | Good | | 30 | 41 | 48 |
| Pinyon-juniper—pinyon, juniper, or both; | Poor | | 75 | 85 | 89 |
| grass understory. | Fair | | 58 | 73 | 80 |
| | Good | | 41 | 61 | 71 |
| Sagebrush with grass understory. | Poor | | 67 | 80 | 85 |
| | Fair | | 51 | 63 | 70 |
| | Good | | 35 | 47 | <mark>55</mark> |
| Desert shrub—major plants include saltbush, | Poor | 63 | 77 | 85 | 88 |
| greasewood, creosotebush, blackbrush, bursage, | Fair | 55 | 72 | 81 | 86 |
| palo verde, mesquite, and cactus. | Good | 49 | 68 | 79 | 84 |

 $^{^{\, 1}}$ $\,$ Average runoff condition, and $I_a,$ = 0.2S. For range in humid regions, use table 2-2c.

Poor: <30% ground cover (litter, grass, and brush overstory).
 Fair: 30 to 70% ground cover.

Good: > 70% ground cover.

 $^{^{\}rm 3}$ $\,$ Curve numbers for group A have been developed only for desert shrub.

| Chapter 2 | Estimating Runoff | Technical Release 55 |
|-----------|-------------------|--------------------------------------|
| | | Urban Hydrology for Small Watersheds |

Antecedent runoff condition

The index of runoff potential before a storm event is the antecedent runoff condition (ARC). ARC is an attempt to account for the variation in CN at a site from storm to storm. CN for the average ARC at a site is the median value as taken from sample rainfall and runoff data. The CN's in table 2-2 are for the average ARC, which is used primarily for design applications. See NEH-4 (SCS 1985) and Rallison and Miller (1981) for more detailed discussion of storm-to-storm variation and a demonstration of upper and lower enveloping curves.

Urban impervious area modifications

Several factors, such as the percentage of impervious area and the means of conveying runoff from impervious areas to the drainage system, should be considered in computing CN for urban areas (Rawls et al., 1981). For example, do the impervious areas connect directly to the drainage system, or do they outlet onto lawns or other pervious areas where infiltration can occur?

Connected impervious areas — An impervious area is considered connected if runoff from it flows directly into the drainage system. It is also considered connected if runoff from it occurs as concentrated shallow flow that runs over a pervious area and then into the drainage system.

Urban CN's (table 2-2a) were developed for typical land use relationships based on specific assumed percentages of impervious area. These CN vales were developed on the assumptions that (a) pervious urban areas are equivalent to pasture in good hydrologic condition and (b) impervious areas have a CN of 98 and are directly connected to the drainage system. Some assumed percentages of impervious area are shown in table 2-2a

If all of the impervious area is directly connected to the drainage system, but the impervious area percentages or the pervious land use assumptions in table 2-2a are not applicable, use figure 2-3 to compute a composite CN. For example, table 2-2a gives a CN of 70 for a 1/2-acre lot in HSG B, with assumed impervious area

of 25 percent. However, if the lot has 20 percent impervious area and a pervious area CN of 61, the composite CN obtained from figure 2-3 is 68. The CN difference between 70 and 68 reflects the difference in percent impervious area.

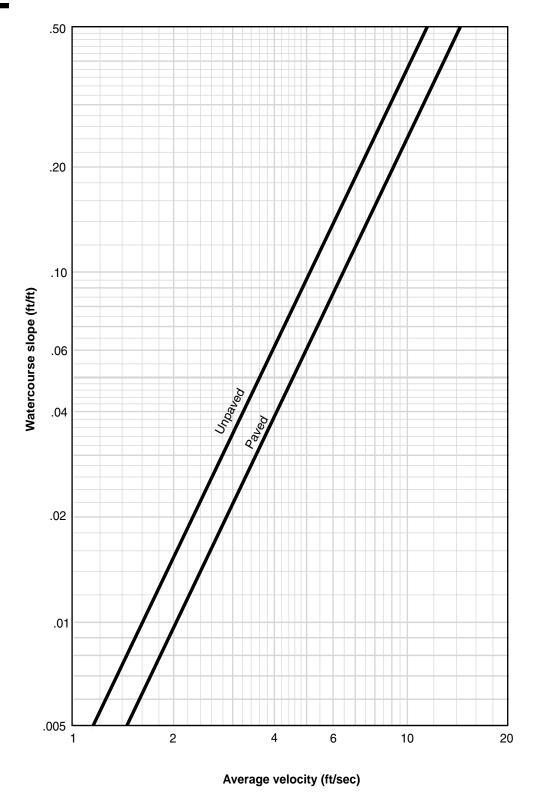
Unconnected impervious areas — Runoff from these areas is spread over a pervious area as sheet flow. To determine CN when all or part of the impervious area is not directly connected to the drainage system, (1) use figure 2-4 if total impervious area is less than 30 percent or (2) use figure 2-3 if the total impervious area is equal to or greater than 30 percent, because the absorptive capacity of the remaining pervious areas will not significantly affect runoff.

When impervious area is less than 30 percent, obtain the composite CN by entering the right half of figure 2-4 with the percentage of total impervious area and the ratio of total unconnected impervious area to total impervious area. Then move left to the appropriate pervious CN and read down to find the composite CN. For example, for a 1/2-acre lot with 20 percent total impervious area (75 percent of which is unconnected) and pervious CN of 61, the composite CN from figure 2-4 is 66. If all of the impervious area is connected, the resulting CN (from figure 2-3) would be 68.

TR 55 Worksheet 3: Time of Concentration (T_c) or Travel Time (T_t)

| Project: | | | Designed By: | Date | : |
|--|--|---------------------------------|----------------------------|------------------------|-----------------|
| Location: | | | Checked By: | Date | : |
| Circle one: | Present | Developed | | | |
| Circle one: | T_c T_t | through subarea | | | |
| | nce for as many a of flow segmen | | w type can be used for eac | h worksheet. Include a | map, schematic, |
| Sheet Flow (A | Applicable to T_{c} | only) | Segment ID | | |
| 1. Surface de | escription (Table | 3-1) | | | |
| 2. Manning's | roughness coef | f., n (Table 3-1) | | | |
| 3. Flow lengt | h, L (total L <u><</u> 10 | 00 ft) | ft | | |
| 4. Two-year 2 | 24-hour rainfall, | P ₂ | in | | |
| 5. Land slope | e, s | | ft/ft | | |
| 6. $T_t = \frac{0.007}{P_2^{0.5} s^0}$ | | Compute T _t | hr | + | = |
| Shallow Conc | cetrated Flow | \$ | Segment ID | | |
| 7. Surface de | escription (paved | d or unpaved) | | | |
| 8. Flow lengt | h, L | | ft | | |
| 9. Watercour | se slope, s | | ft/ft | | |
| 10. Average v | elocity, V (Figur | e 3-1) | ft/s | | |
| 11. $T_t = L$ 3600 | _ | Compute T _t | | + | = |
| Channel Flow | <u>'</u> | S | egment ID | | |
| 12. Cross sec | ctional flow area | ı, a | ft ² | | |
| 13. Wetted po | erimeter, P _w | | ft | | |
| 14. Hydraulic | radius, r = <u>a</u> (P _w | Compute r | ft | | |
| 15 Channel | •• | | ft/ft | | \neg |
| | • | | | | _ |
| 17. V = <u>1.49 r</u> | | oeff., n | | | _ |
| | | Compute V | 175 | | |
| n 40. Elavvlana | | | <i>t</i> , | | \neg |
| _ | | | | | |
| 19. $T_t = L$ 3600 | | mpute T _t | hr | + | = |
| 20. Watershe | ed or subarea T _c | or T_t (add T_t in steps 6, | 11, and 19 | | hr |

Figure 3-1 Average velocities for estimating travel time for shallow concentrated flow



Sheet flow

Sheet flow is flow over plane surfaces. It usually occurs in the headwater of streams. With sheet flow, the friction value (Manning's n) is an effective roughness coefficient that includes the effect of raindrop impact; drag over the plane surface; obstacles such as litter, crop ridges, and rocks; and erosion and transportation of sediment. These n values are for very shallow flow depths of about 0.1 foot or so. Table 3-1 gives Manning's n values for sheet flow for various surface conditions.

Table 3-1 Roughness coefficients (Manning's n) for sheet flow

| Surface description | | | |
|-------------------------------------|-------|--|--|
| | | | |
| Smooth surfaces (concrete, asphalt, | | | |
| gravel, or bare soil) | 0.011 | | |
| Fallow (no residue) | | | |
| Cultivated soils: | | | |
| Residue cover ≤20% | 0.06 | | |
| Residue cover >20% | 0.17 | | |
| Grass: | | | |
| Short grass prairie | 0.15 | | |
| Dense grasses 2/ | 0.24 | | |
| Bermudagrass | 0.41 | | |
| Range (natural) | | | |
| Woods:3/ | | | |
| Light underbrush | 0.40 | | |
| Dense underbrush | 0.80 | | |

¹ The n values are a composite of information compiled by Engman (1986)

For sheet flow of less than 300 feet, use Manning's kinematic solution (Overtop and Meadows 1976) to compute T_t :

$$T_{t} = \frac{0.007(nL)^{0.8}}{(P_{2})^{0.5} s^{0.4}}$$
 [eq. 3-3]

where:

 $T_t = \text{travel time (hr)},$

n = Manning's roughness coefficient (table 3-1)

L = flow length (ft)

 P_2 = 2-year, 24-hour rainfall (in)

s = slope of hydraulic grade line

(land slope, ft/ft)

This simplified form of the Manning's kinematic solution is based on the following: (1) shallow steady uniform flow, (2) constant intensity of rainfall excess (that part of a rain available for runoff), (3) rainfall duration of 24 hours, and (4) minor effect of infiltration on travel time. Rainfall depth can be obtained from appendix B.

Shallow concentrated flow

After a maximum of 300 feet, sheet flow usually becomes shallow concentrated flow. The average velocity for this flow can be determined from figure 3-1, in which average velocity is a function of watercourse slope and type of channel. For slopes less than 0.005 ft/ft, use equations given in appendix F for figure 3-1. Tillage can affect the direction of shallow concentrated flow. Flow may not always be directly down the watershed slope if tillage runs across the slope.

After determining average velocity in figure 3-1, use equation 3-1 to estimate travel time for the shallow concentrated flow segment.

Open channels

Open channels are assumed to begin where surveyed cross section information has been obtained, where channels are visible on aerial photographs, or where blue lines (indicating streams) appear on United States Geological Survey (USGS) quadrangle sheets.

Manning's equation or water surface profile information can be used to estimate average flow velocity. Average flow velocity is usually determined for bankfull elevation.

² Includes species such as weeping lovegrass, bluegrass, buffalo grass, blue grama grass, and native grass mixtures.

 $^{^3}$ When selecting n, consider cover to a height of about 0.1 ft. This is the only part of the plant cover that will obstruct sheet flow.

Tr 55 Worksheet 4: Graphical Peak Discharge Method

| Project: | | Designed By: | | Date: | | | | |
|---|-------|--------------------------|----------|----------|--|--|--|--|
| Location: | | Checked By: | | Date: | | | | |
| Circle one: Present Developed | | | | | | | | |
| 1. Data: | | | | | | | | |
| Drainage area A _m = | mi² | ² (acres/640) | | | | | | |
| Runoff curve number CN = (From Worksheet 2) | | | | | | | | |
| Time of concentration T _c = hr (From Worksheet 3) | | | | | | | | |
| Rainfall distribution type = (II, III, DMVIII) | | | | | | | | |
| Pond and swamp areas spread throughout watershed = percent of A _m (acres or mi ² covered) | | | | | | | | |
| | | Storm #1 | Storm #2 | Storm #3 | | | | |
| 2. Frequency | . yr | | | | | | | |
| 3. Rainfall, P (24-hour) | in | | | | | | | |
| Initial abstraction, I _a (Use CN with Table 4-1.) | in | | | | | | | |
| 5. Compute I _a /P | | | | | | | | |
| 6. Unit peak discharge, q _u c (Use T _c and I _a /P with exhibit 4) | sm/in | | | | | | | |
| 7. Runoff, Q (From Worksheet 2) | in | | | | | | | |
| Pond and swamp adjustment factor, F_p (Use percent pond and swamp area with Table 4-2. Factor is 1.0 for zero percent pond and swamp area.) | in | | | | | | | |
| 9. Peak discharge, q _p (Where q _p = q _u A _m QF _p) | . cfs | | | | | | | |

APPENDIX D SOIL MAP

